

## GENERAL INFORMATION

### General

Presented in these load tables are maximum uniformly distributed specified loads.

### ❖ Limit States Design (LSD)

**Strength** - Limit States Design principles were used in the development of the load tables in accordance with CSA-S136-07, North American Specification for the Design of Cold Formed Steel Structural Members and the National Building Code of Canada. The factored resistance under consideration,  $\phi R$ , must be equal to or greater than the effect of the factored loads, i.e.,

$$\phi R \geq \text{Effect of Factored Loads.}$$

Hence, a short calculation must be carried out to compute the specified live load. See example.

**Serviceability** – Maximum specified deflection loads given in the tables must be compared with their respective specified live loads.

### ❖ Steel

**Specification** - Conforms to ASTM A653M grade 230; Yield strength 230 MPa (33 ksi) and tensile strength of 310 MPa (45 ksi).

**Finishes** – ZF 075 (A25) or Z275 (G90). For heavier galvanizing, refer to ASTM A653-A653M.

### ❖ Design Considerations

**Strength** - The maximum uniformly distributed specified load obtained from the load table must be equal to or greater than (*Specified live load + 0.833 times the specified dead load*).

Where  $0.833 = 1.25/1.5$ .

**Conservative Strength Approach** - The maximum uniformly distributed specified load obtained from the load table must be equal to or greater than (*Specified live load + specified dead load*).

**Web Crippling Check** – If  $n/t > 210$ , use  $n/t = 210$ .

**Serviceability (Deflection)** – The effective moment of inertia for deflection determination has been calculated at an assumed specified live load stress of  $0.6F_y$ .

## EXAMPLE

### 3" ROOF DECK (IMPERIAL)

#### Given:

- ❑ Two-span continuous,  $L = 12$  ft each span
- ❑ Deck thickness,  $t = 0.036$  in
- ❑ L/180 deflection limit (data in tables are based on L/240)
- ❑ Bearing length,  $n = 3$  in. ( $n/t = 83.3$ )
- ❑ Specified loads
  - 1) Dead loads (DL)
    - a) deck 2.6 psf
    - b) superimposed 8.2 psf $DL = 10.8$  psf
  - 2) Live load (LL) $LL = 40$  psf

#### Solution:

##### ▪ Strength

- 1) Specified loads $[LL + 0.833 DL]$  $[40 + 0.833 (10.8)] = 49.0$  psf
- 2) Maximum specified load (from Table under "S") is 52 psf  
Since  $52 > 49.0 \therefore$  OK
- 3) Check end web crippling ( $n = 3$  in.)
  - a) Specified end reaction $0.375(49.0)12 = 221$  lb/ft
  - b) Maximum specified end reaction (from Section Property Table) $P_e = P_{e1} + P_{e2} \sqrt{n/t}$  $P_e = 216 + 54.0 \sqrt{3/0.036} = 709$  lb/ft  
Since  $709 > 221 \therefore$  OK
- 4) Check interior web crippling ( $n = 3$  in.)
  - a) Specified interior reaction $1.25(49.0)12 = 735$  lb/ft
  - b) Maximum specified interior reaction (from Section Property Table) $P_i = P_{i1} + P_{i2} \sqrt{n/t}$  $P_i = 409 + 69.5 \sqrt{3/0.036} = 1043$  lb/ft  
Since  $1043 > 735 \therefore$  OK

##### ▪ Deflection

From Table under "D" (L/240) = 88 psf  
For L/180, multiply 88 by  $240/180 = 117$  psf.  
Since  $117 > 40 \therefore$  OK



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